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Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

[jets] Submission Acknowledgement from Journal of Engineering and Technological Sciences

2 pesan

Prof.Dr. Tjandra Setiadi <jets@lppm.itb.ac.id>

7 Februari 2020 pukul 20.18

Kepada: Dr Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

Dear Dr Ni Nyoman Kencanawati,

This is to confirm that the manuscript, "Evaluation of Building Seismic Design Parameters in Mataram City Using Lombok Earthquake 2018 Ground Motion", has been received for consideration in the Journal of Engineering and Technological Sciences.

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Thank you for submitting your manuscript to the Journal of Journal of Engineering and Technological Sciences. Should you have any questions, please feel free to contact our office.

With kind regards,

Prof.Dr. Tjandra Setiadi
Journal of Engineering and Technological Sciences

Journal of Engineering and Technological Sciences
<http://journals.itb.ac.id/index.php/jets>

Prof.Dr. Tjandra Setiadi <jets@lppm.itb.ac.id>

7 Februari 2020 pukul 20.36

Kepada: Dr Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

Dear Dr Ni Nyoman Kencanawati,

This is to confirm that the manuscript, "Evaluation of Building Seismic Design Parameters in Mataram City Using Lombok Earthquake 2018 Ground Motion", has been received for consideration in the Journal of Engineering and Technological Sciences.

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Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

Revision 13104 [jets] Editor Decision from Journal of Engineering and Technological Sciences

4 pesan

jets@lppm.itb.ac.id <jets@lppm.itb.ac.id>
Kepada: nkencanawati@unram.ac.id
Cc: itbjournal <itbjournal@gmail.com>

22 April 2020 pukul 11.32

Dear Dr Ni Nyoman Kencanawati,

We have reached a decision regarding your submission to Journal of Engineering and Technological Sciences, "Evaluation of Building Seismic Design Parameters in Mataram City Using Lombok Earthquake 2018 Ground Motion".

Our decision is: Revision Required.

If you are able to correct the paper taking into account all of the points raised in the referees' report, I would be willing to arrange for the paper to be reviewed again. Revisions should be submitted within 14 days.

Besides the referees' report as attached in this email, see also the "Reviewer Uploaded Files" in journal's website to read the additional comments from reviewers (if any).

If you choose to revise your manuscript it will be due into the Editorial Office by 6 May 2020.

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To upload your revision's comments and other additional files (if any), click on "Add a Supplementary File" under Summary tab.

Thank you, and we look forward to receiving your revised manuscript.

With kind regards,

Prof.Dr. Tjandra Setiadi
Journal of Engineering and Technological Sciences
Institut Teknologi Bandung
jets@lppm.itb.ac.id

Reviewer A:

Is the paper content original?:

Yes

Does the paper title represent its content?:

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Does the abstract reflect the paper content?:

Yes

Do the keywords indicate the scope of the research?:

Yes

Is the research methodology or the approach of the problem solving clearly described?:

Yes

Do the data presentation and interpretation valid and reasonable?:

Yes

Do the use of tables and figures help to clarify the explanation?:

Yes

Have the discussion and/or analysis been relevant with the results of the study?:

Yes

Are the references used relevant?:

Yes

Contribution to science::

Very Good

Originality:

Very Good

Systematic::

Good

Language::

Good

Writing Accuracy::

Good

Comments to Author::

Refer to the attached file for details of the pointed out points.

If the probability is 2% with a 50-year probability, is the confirmation cycle 2500 years? Does that mean that an event occurs once every 2500 years?

Is the title more appropriate for Investigation than Evaluation?

Units are not shown in italics. (Second; s)

Since g of the gravitational acceleration is different from g of the unit (gram) of the mass, it is necessary to change the font face.

There may be other grammatical or notational problems. Therefore, it is necessary to perform proof-reading.

Reviewer attach the other file as reference for revision. Please see carefully.

Reviewer C:

Is the paper content original?:

Yes

Does the paper title represent its content?:

Yes

Does the abstract reflect the paper content?:

Yes

Do the keywords indicate the scope of the research?:

Yes

Is the research methodology or the approach of the problem solving clearly described?:

Yes

Do the data presentation and interpretation valid and reasonable?:

Yes

Do the use of tables and figures help to clarify the explanation?:

Yes

Have the discussion and/or analysis been relevant with the results of the study?:

Yes

Are the references used relevant?:

See Comment

Contribution to science::

Good

Originality:

Good

Systematic::

Very Good

Language::

Good

Writing Accuracy::

Very Good

Comments to Author::

literature should be added to the more recent, in addition to journals also books about the earthquake and articles that are more relevant to lombok earthquake.

 **Author's_Respond_jets.docx**
22K

Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>
Kepada: "Prof.Dr. Tjandra Setiadi" <jets@lppm.itb.ac.id>

22 April 2020 pukul 12.39

Dear Prof Setiadi,

I would like to say thank you for your email.

I will revise the manuscript according to the comments and suggestions and submit back the revised version to JETS online system according to the specified time.

Sincerely yours

Ni Nyoman Kencanawati, Ph.D

[Kutipan teks disembunyikan]

Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

23 April 2020 pukul 12.42

Kepada: I G P Suta Wijaya <gpsutawijaya@unram.ac.id>

[Kutipan teks disembunyikan]



Author's_Respond_jets.docx

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Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

25 April 2020 pukul 13.57

Kepada: "Prof.Dr. Tjandra Setiadi" <jets@lppm.itb.ac.id>

Dear Prof. Setiadi,
Editor in Chief of JETS

I would like to inform you that I have submitted the revision file along with the supplementary file (author's response form) in the system successfully. However, when I try to attach another file as a supplementary file which is required from Reviewer A (plagiarism checker), the file can not be uploaded. Therefore, I attach the supplementary file required from Reviewer A through this email. Thank you for your attention.

Sincerely yours,

Ni Nyoman Kencanawati

[Kutipan teks disembunyikan]



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TABLE Author's Respond Form

| | |
|-----------------|---|
| Paper ID | : 13104 |
| Title | : Evaluation of Building Seismic Design Parameters in Mataram City Using Lombok Earthquake 2018 Ground Motion |
| Authors | : Ni Nyoman Kencanawati, Didi Supriyadi Agustawijaya & Rian Mahendra Taruna |
| Submission date | : February, 7 th 2020 |
| Revision date | : February, 23 rd 2020 |
| Round | : 1 |
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Round : 1

| Reviewers' Evaluation | Details of Author's Respond | Modification |
|---|---|---|
| Reviewer A Comments | Author's Respond | |
| 1. If the probability is 2% with a 50-year probability, is the confirmation cycle 2500 years? Does that mean that an event occurs one every 2500 years? | <p>1. SNI-1726:2012 specifies the ground motion values with a 2% probability of being exceeded in 50 years. It means that the spectral acceleration has a return period of 2475 years as this equation shows.</p> <p>The return period of earthquake with 2% probability of being exceeded in a 50 years building expected life time, can be estimated by:</p> $R_n = \left\{ 1 - \left(1 - \frac{1}{T_R} \right)^N \right\} \times 100\%$ <p>Where: R_N is the probability of an earthquake occurring during the building expected life (2%). N is the expected life of a building (50 years). Thus, it gives the return period (T_R) = 2475 years.</p> | |
| 2. Is the title more appropriate for Investigation than Evaluation? | 2. Yes, we accept the reviewer's suggestion | 1. The title has been changed to: An Investigation of Building Seismic Design Parameters in Mataram City Using Lombok Earthquake 2018 Ground Motion |

| | | |
|---|---|---|
| 3. Units are not shown in italics. (Second; s) | 3. Yes, we revised the manuscripts according to reviewer's suggestion | 2. The unit 's' has been replaced by a normal style. All the unit of 's' in entire text is highlighted by yellow color to facilitate the checking process. |
| 4. Since g of the gravitational acceleration is different from g of the unit (gram) of the mass, it is necessary to change the font face. | 4. Yes, we accept the reviewer's suggestion | 3. The gravitational acceleration 'g' has been written as your suggestion using italics serif letter. All the changes are highlighted in the text by yellow color to help the checking process. |
| 5. There may be other grammatical or notational problems. Therefore, it is necessary to perform proof-reading | 5. Yes, we agree with the reviewer's suggestion. | 4. To consider the reviewer's suggestion, we used the Grammarly Checker and Turnitin tool for revising the manuscript. In addition, the result of Turnitin that indicates the plagiarism score is attached on the supplementary file. |
| 6. Reviewer attach the other file as reference for revision. Please see carefully. | 6. Please see the entire revised manuscript that the suggestion has been accommodated and all the correction has been made to the text with highlighted yellow color and red typed. | |
| | | |
| Reviewer C Comments | Author's Respond | |
| 1. Literature should be added to the more recent, in addition to journals also books about the earthquake and articles that are more relevant to lombok earthquake. | 1. Yes, we accept the reviewer's suggestion. | <p>1. Books and journals related to Lombok 2018 earthquake have been added to enrich the literature and to improve the analysis.</p> <p>Books: Two books related to earthquake engineering and earthquake resistant's structures have been included, therefore three books are referred in the manuscript.</p> <p>a. The book related to basic earthquake engineering to strengthen the review on the ground motion is provided in: [21] A. S. Elnashai and L. Di Sarno, <i>Fundamentals of earthquake engineering</i>. Wiley New York, 2008.</p> <p>b. The book related to earthquake resistant structures to improve the analysis of the building design paramaters is provided in: [22] V. Giouncu and F. M. Mazzolani, <i>Earthquake Engineering for Structural Design</i>. New York, USA:</p> |

Roudledge, 2013.

- [23] S. K. Duggal, *Earthquake resistant design of structures*. Oxford university press New Delhi, 2007.

Journals

There are 5 journals that published in most recent years have been cited in the manuscript.

a. Journals related to damage after earthquake:

- [3] A. Pomonis, J. Daniell, R. Gunasekera, A. Schaefer, and J.-U. Skapski, "The 2018 Lombok and Palu, Indonesia, earthquakes: Loss data uncertainties, cascades and implications.," in *Geophysical Research Abstracts*, 2019, vol. 21.
- [4] Asmirza, M Sofian, "Analysis of school damage due to Lombok earthquake on August 2018," *E3S Web Conf.*, vol. 156, p. 5019, 2020.

b. Journals related to the occurrence of Lombok earthquake:

- [18] P. Supendi *et al.*, "Relocated aftershocks and background seismicity in eastern Indonesia shed light on the 2018 Lombok and Palu earthquake sequences," *Geophys. J. Int.*, vol. 221, no. 3, pp. 1845–1855, 2020.
- [19] Agustan, R. N. Hanifa, Y. Anantasena, M. Sadly, and T. Ito, "Ground Deformation Identification related to 2018 Lombok Earthquake Series based on Sentinel-1 Data," *(IOP) Conf. Ser. Earth Environ. Sci.*, vol. 280, p. 12004, Aug. 2019.

c. Journal related to the results of the post-Lombok earthquake study is needed to better disaster mitigation for the future:

- [27] F. Ramdani, P. Setiani, and D. A. Setiawati, "Analysis of sequence earthquake of Lombok Island, Indonesia," *Prog. Disaster Sci.*, vol. 4, p. 100046, 2019.

Please see all the additional journals suggested in cyan highlighted and red typed in the manuscript.



1 **An Investigation** of Building Seismic Design Parameters in
2 **Mataram City Using Lombok Earthquake 2018 Ground**
3 **Motion**

4 **Ni Nyoman Kencanawati¹, Didi Supriyadi Agustawijaya¹ & Rian Mahendra**
5 **Taruna²**

6 ¹ Department of Civil Engineering, Mataram University, Jl. Majapahit No. 62, 83115
7 Mataram, Indonesia

8 ² Meteorology, Climatology and Geophysics Agency, Jl. Tgh. Ibrahim Khalidi, 83362
9 Lombok Barat, Indonesia
10 Email: nkencanawati@unram.ac.id

11
12

13 **Novelty:** The strong earthquake in Lombok in 2018 caused an increase in
14 spectral acceleration compared to what is stated in Indonesia's current earthquake
15 code. As a result, changes affect building design parameters. This paper showed
16 that the seismic response factor of the building increased by 10.782% and
17 13.168% on medium and soft soil, respectively, compared to that of the current
18 code. It also recommends that seismic codes need to be improved to provide
19 better preparedness for future seismic risk reduction.

20

Highlight:

- 21 • Spectral acceleration using the Lombok Earthquake 2018 is analyzed,
22 • **The spectral acceleration is greater than the existing seismic code acceleration,**
23 • The seismic response coefficient is higher than the existing seismic codes,
24 • Existing seismic building standards need to be improved.

25

26 **Abstract.** Mataram is the capital of West Nusa Tenggara. West Nusa Tenggara
27 is made up of two islands, Lombok and Sumbawa. The 2018 earthquake on
28 Lombok has undoubtedly affected spectral acceleration. This is an important
29 factor to be addressed in structural design. **Short period spectral acceleration, S_s**
30 **increases 18.323% compared to the value listed in the seismic code SNI**
31 **1976:2012 corresponding to the 2500 return period.** However, even if the S_s
32 value increases, the design category of the building does not change and remains
33 in the D category. In general, the acceleration value in this study is found
34 relatively greater than that of the existing code for periods of less than 0.462 s
35 for site class D, and in periods of less than 0.830 s in site class E. In addition, the
36 seismic response coefficient, C_s , for medium soil, it increases by 10.782%
37 compared to the C_s calculated using of the current code. This effect is more
38 severe in soft soil areas where the increase reaches 13.168%. Improving existing
39 codes with seismic design parameters for new buildings affected by the ground
40 motion of recent strong earthquakes will lead to more preparedness and will be
an important part of local disaster risk reduction.

41 **Keywords:** *Lombok earthquake 2018; spectral acceleration; seismic design*
42 *parameters, seismic code*

43 **1 Introduction**

44 The West Nusa Tenggara region is an area of high seismic activity, surrounded
45 by two active seismic sources. In the south is the subduction zone of the Indo-
46 Australia Sea Plate, and in the north is the back-arc thrust zone. According to
47 the National Institutes of Meteorology, Climatology, and Geophysics
48 (Indonesian: Badan Meteorologi, Klimatologi, dan Geofisika (BMKG)), a
49 magnitude 6.2 magnitude earthquake on June 9, 2016, occurred in Mataram and
50 Central Lombok and caused some damages. Later, in 2017, several quakes were
51 hit at the scale of II-III MMI in Mataram City, **as written by Taruna et al. in [1].**
52 In addition, officials reported that there were 3699 earthquake events in 2018
53 and 215 events were felt. **One of a series of Lombok earthquakes on August 5,**
54 **2018, with a magnitude of 7.0, caused severe damage to a number of buildings**
55 **and houses, even some collapsed in Lombok area, including the city of**
56 **Mataram, as announced by BMKG in [2] and published by Pomonis in [3] and**
57 **Asmirza in [4].**

58 In the past, some countries have changed their seismic codes after large
59 earthquakes that caused various damage to structures and buildings. As studied
60 by Okamura in [5] and Karakostas et al. in [6], the seismic code has been
61 improved with a new response spectrum affected by recent ground acceleration.
62 **Similarly, Indonesia has the current code for seismic structures, namely SNI**
63 **1726: 2012 in [7].** The ground motion is calculated with a 2% probability of
64 being exceeded within 50 years. **The return period of the spectral acceleration is**
65 **2500 years.** It replaces SNI 1726:2002 in [8]. SNI 1726:2002 provides spectral
66 acceleration by dividing all areas of Indonesia into six seismic zones. The
67 current Seismic Building Code has been improved by providing spectrally
68 accelerated design values at each coordinate point in Indonesia. Seismic

69 acceleration maps are also attached for spectral accelerations at $T = 0$ s (PGA),
70 $T = 0.2$ s (short period), and $T = 1$ s (long period).

71 The previous seismic design code, SNI 1726:2002, has been reviewed by
72 Sengara et al. [9]. In addition, compared to the previous seismic code SNI
73 1726:2002 presented by Arfiadi and Satyarno in [10], some of the Indonesian
74 short period design spectral acceleration, S_{DS} , have a significant increase in
75 current seismic code SNI 1726:2012. Significant increases in S_{DS} are evident in
76 some areas, such as Aceh, Palu, Yogyakarta, and Padang, which were affected
77 by major earthquakes during the time when the previous code was applied.
78 Therefore, the values have been modified in the current code. In Palu, S_{DS} has
79 the largest increase, with 116.7%, 85.7% and 41.2% in hard, medium, and soft
80 soils, respectively. This region was hit by a magnitude 7.7 earthquake in 2008,
81 and the 2012 seismic code changed the spectral acceleration. In the other prone
82 areas mentioned above, the S_{DS} of three types of soil has risen from 10% to
83 80%. Conversely, Lombok did not show significant seismic activity during that
84 period. Therefore, the 2012 seismic code shows little change in acceleration.

85 To obtain a new spectral acceleration that includes the site amplification factor,
86 strong ground motions after the earthquake must be considered. This is
87 compared to the existing spectral acceleration provided by the existing code to
88 make sure there is a sufficient design to face strong earthquakes that may occur
89 in the future, as reported by Panzera et al. in [11] and Mase, Likitlersuang, and
90 Tobita in [12]. Furthermore, the evaluation of seismic codes after earthquakes
91 has been carried out in some countries. The earthquake code has been updated
92 to consider the recent ground acceleration due to the earthquake. In addition to
93 the response spectrum, details of the structural design have been improved
94 further as given by Okamura in [5], Karakostas et al. in [6], Sezen et al. in [13],

95 Ergün, Kiraç, and Bacsaran in [14], Mosleh et al. in [15] and Baros and Santa-
96 Maria in [16].

97 The analysis describes the seismic hazard in Mataram city using seismic data up
98 to 2017 with a 2% probability of being exceeded in 50 years (return period of
99 2500 years). The short period of bedrock acceleration S_S ($T = 0.2$ s) and the
100 long period of bedrock acceleration S_1 ($T = 1$ s) were reported to be in the range
101 of 0.37-0.45 g ($g = 9.81\text{m/s}^2$) and 0.16-0.18 g, respectively. Furthermore, the
102 values of S_S and S_1 in the northern region of Mataram are higher than those in
103 the southern region of Mataram. This is due to the superiority of the Back Arc
104 Thrust activity in northern Lombok as given by Taruna in [1].

105 The 2018 earthquake on Lombok is an important consideration in spectral
106 acceleration. This is an important factor to be addressed in structural design.
107 Improving the calculation of parameters will lead to the reproduction of the
108 structural design under seismic loading, which is part of disaster risk reduction.
109 It could potentially save millions of people and reduce major risks in the region
110 in the future. Therefore, a new spectral acceleration needs to be approached
111 using the recent 2018 seismic data, which applies to some seismic parameters
112 that will help better seismic structures.

113 2 Related Research and Theory

114 According to Agustawijaya, Sulistyono, and Elhuda in [17], Lombok is
115 classified as moderate to high seismic activity. Before the strong earthquake of
116 2018, this study states that the South Subduction Megathrust and the North
117 Back-Arc Thrust have established the tectonic pattern of Lombok Island as an
118 effect of compression between the Australian continental plate and Eurasia.
119 Then in 2018, a series of earthquakes occurred in North Lombok which was
120 triggered by Flores back arc thrust. The ground motion initially began on July 28

121 with an M_w 6.4 earthquake in the northern part of Lombok. Aftershocks with
122 $M_w < 5$ followed the first earthquake a few hours later. On August 5, a larger
123 shock of M_w 7.0 occurred. Then, in the following two weeks, an M_w 6.9
124 earthquake hit the island on August 19, 2018. The sequences of Lombok ground
125 motions have been studied in detail by some researchers in [18], [19].

126 As reported by Marjiyono in [20], in general, the plains of Mataram City are
127 dominated by alluvial deposits with sandy materials, either product of the
128 eastern river process or marine products of the West Side. The alluvium fills an
129 ancient form in the form of a basin in the western part of Mataram. Physically,
130 alluvial sediments are soft and are indicated by low shear wave velocity values.
131 This condition is potentially for areas that experience such wave amplification
132 during an earthquake [21]. In addition, the average measurement of shear wave
133 velocity v_s shows the value range of 135-201 m/s in Mataram City. This value is
134 included in site class D (SD) and site class E (SE) of the current building
135 seismic code.

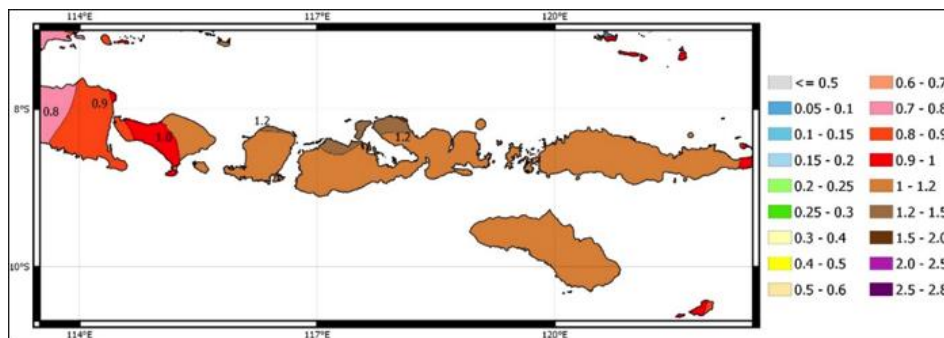
136 S_s and S_1 must be determined at $T = 0.2$ s and $T = 1$ s, respectively, provided in
137 the ground motion map of the SNI 1726:2012 code, and may exceed 2% in 50
138 years. By multiplying the S_s and S_1 values by the amplification factor from each
139 site class, the short-period, S_{MS} , and long-term S_{M1} surface maximum ground
140 acceleration can be calculated directly [22], [23]. The amplification factor F_a is
141 related to the acceleration of the short-period S_s , while the amplification factor
142 associated with S_1 is F_v . Furthermore, S_{MS} and S_{M1} values are used to calculate
143 design spectral acceleration parameters for short period, S_{DS} and long period
144 S_{D1} , as described by in SNI 1726:2012 in [7].

145 **3 Method**

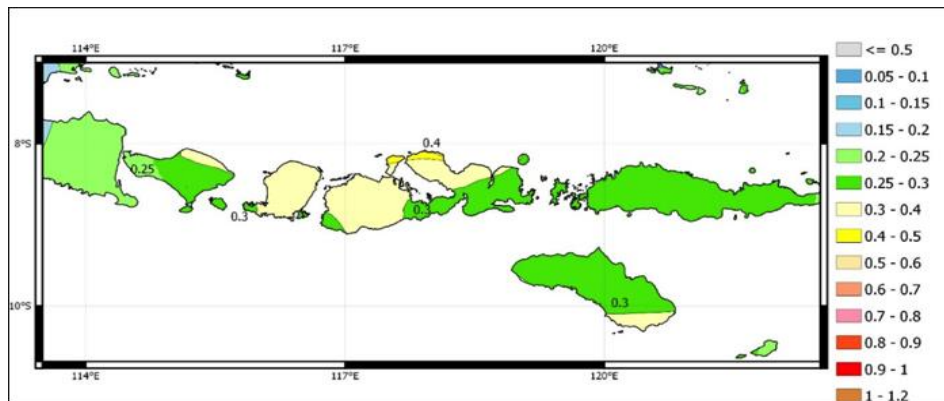
146 **3.1 Ground Acceleration Data**

147 Ground acceleration data is based on previous work studied by Taruna,
 148 Agustawijaya, and Kencanawati in [24]. Earthquake data was obtained from
 149 Engdahl ISC (EHB), USGS, and BMKG in 1922-2018. The data was taken at
 150 coordinates of the latitude of 7° - 12° and longitude of 113.5° - 122.5° or about
 151 300 km from Mataram City with magnitude, $M_w \geq 4.5$. This magnitude is
 152 assumed to be the standard for earthquakes related to the risk of seismic
 153 disasters. In this study, the values of peak ground motion in the bedrock soil
 154 layer from the previous study are used. Ground motion or maximum
 155 acceleration will be adopted as the parameters used in this study. These
 156 parameters are S_S and S_1 which are related to the technical design of earthquake-
 157 resistant structures, as shown in Figs. 1-2.

158



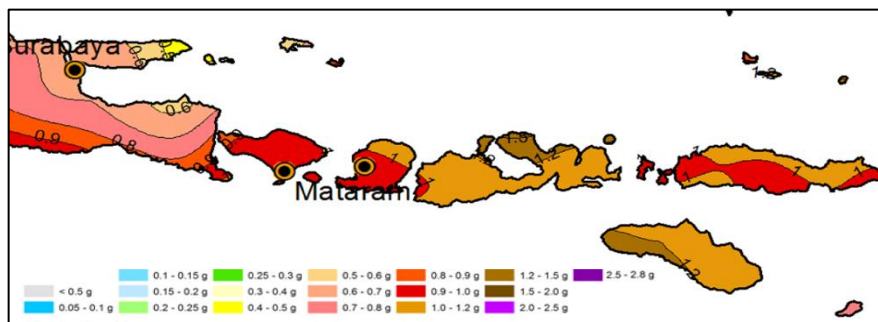
159 **Figure 1** Spectral Acceleration at $T = 0.2$ s in bedrock with a probability exceeding 2%
 160 in 50 years for Bali-West Nusa Tenggara region, Taruna in [24]



161

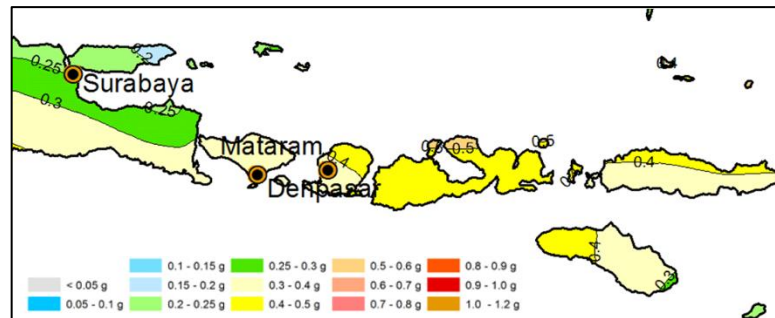
162 **Figure 2** Spectral Acceleration at $T = 1$ s in bedrock with a probability exceeding 2% in
 163 50 years for Bali-West Nusa Tenggara region, Taruna in [24]

164 As shown in Fig. 1, in most Lombok regions, S_S values range from 1-1.2 g, and
 165 in North Lombok, the values are over 1.2 g. S_S values tend to be larger than the
 166 0.9-1.2 g values for Lombok, calculated in SNI 1726:2012 (Fig. 3). This could
 167 be caused by the large earthquake data used in previous studies, especially the
 168 increase in the 2018 Lombok earthquake series. On the other hand, the
 169 maximum acceleration of S_1 is 0.25 to 0.4 g. The S_1 values in the Lombok region
 170 used in this study are lower than those of SNI 1726:2012 (Fig. 4). In SNI
 171 1726:2012, Lombok's S_1 values range from 0.3 to 0.5 g, with the maximum seen
 172 in the north.



173

174 **Figure 3** Spectral Acceleration at $T = 0.2$ s in bedrock with a probability exceeding 2%
 175 in 50 years for the Bali-West Nusa Tenggara region, SNI 1726:2012 in [7]



176

177 **Figure 4** Spectral Acceleration at $T = 1$ s in bedrock with a probability exceeding 2% in
 178 50 years for the Bali-West Nusa Tenggara region, SNI 1726:2012 in [7]

179 3.2 Equivalent Lateral Load Factor

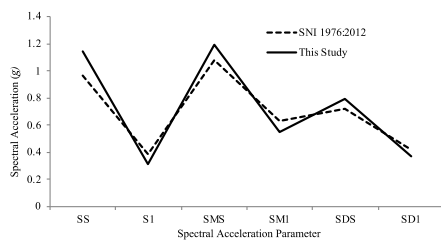
180 The dynamic properties of seismic loads are simplified to horizontal forces with
 181 an equivalent lateral load procedure. For the analysis, the seismic response
 182 coefficient C_s is determined. SNI 1726:2012 provides instructions for obtaining
 183 C_s . It depends on spectral acceleration, S_{DS} and S_{DI} values and parameters such
 184 as seismic design category, importance factor, structural fundamental period,
 185 response modification factor, etc.

186 4 Result and Discussion

187 4.1 Spectral Acceleration

188 As shown in Figs. 5-6, the strong earthquake in Lombok in 2018 increased the
 189 spectral acceleration S_s of Mataram by 1.143 g. This value represents the
 190 location of Mataram latitude: -8.5606 and longitude: 116.0707. This is an
 191 increase of 18.323% from the value listed in SNI 1976:2012. Approximately the
 192 same increase as the spectral acceleration in Padang City when provided in the
 193 previous seismic code compared to the current code (SNI 1976:2012). This is
 194 because Padang experienced a major earthquake in 2009, the transition period
 195 between the previous code and the current code. The following Indonesian
 196 seismic code assumed that the acceleration of Mataram would be potentially

197 higher due to the 2018 Lombok earthquake, as this study shows. Meanwhile,
 198 Sharma in [25] reported that after the Nepal earthquake (M_w 7.9) ground
 199 motion, existing spectral accelerations were still applicable to seismic structural
 200 engineering design.



201

Figure 5 Spectral acceleration parameters in medium soil

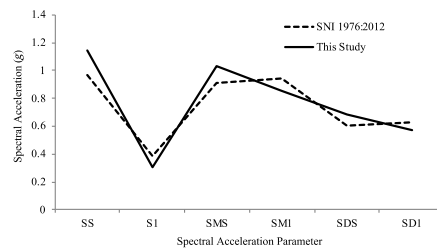


Figure 6 Spectral acceleration parameters in soft soil

202 Figure 5-6 also shows the spectral acceleration of the maximum considered and
 203 design basis earthquakes on the surface at $T = 0.2$ s (S_{MS} and S_{DS}) and $T = 1$ s
 204 (S_{M1} and S_{D1}) on medium soil (Fig. 5) whereas the parameters for soft soil is
 205 illustrated in Fig. 6. The acceleration of the surface is calculated for site class D
 206 (SD) and site class E (SE) because Mataram city is made up of medium and soft
 207 soils as given by Marjiyono in [20]. The spectral acceleration provided by SNI
 208 1726: 2012 is also shown for comparison.

209 Contrary to the acceleration of $T = 0.2$ s, the acceleration of $T = 1$ s used in this
 210 study is smaller than that of SNI 1726:2012 because the constant attenuation
 211 equation is not the same between S_S and S_1 . Furthermore, theoretically, S_1 is a
 212 long period spectrum affected by far-field earthquakes. **On the other hand**, this
 213 study is more dominant near earthquakes.

214 4.2 Building Seismic Design Category

215 Considering the ground motion of recent earthquakes, the S_{DS} and S_{D1} values for
 216 Mataram are 0.795 g and 0.367 g for medium soil and 0.686 g and 0.569 g for

217 soft soil, respectively. According to SNI 1976:2012, buildings at sites with an
 218 S_{DS} greater than 0.5 g are designed as D categories for all risk categories I-IV
 219 (shown in bold in Table 1). Similarly, for S_{D1} values, as shown in Table 2,
 220 Mataram has values greater than 0.2 g in both medium and soft soils. Therefore,
 221 it is included in the D seismic design category (shown in bold in Table 2). S_{DS}
 222 values exceed 0.5 g and S_{D1} values exceed 0.2 g. This is similar to the value in
 223 the current code. Thus, even though the results of the study's spectral
 224 acceleration appear larger than those present in the current seismic code, there is
 225 no change in the seismic design category between the current seismic code and
 226 the results of this study.

227 The D-design seismic category is intended for structures built in the sites which
 228 to be potential for severe and damaging earthquakes, but not located close to
 229 major faults. As given by Giounou and Mazolani in [22] dan Duggal in [23],
 230 structures on poor soils generally fall into the D class for seismic design.
 231 According to Sharma et al. in [25] as mentioned above, there is no change in the
 232 spectral acceleration between the existing code and the spectral acceleration
 233 after the Nepal earthquake, however, it is recommended to implement the
 234 existing code to develop mitigation strategies and structures.

Table 1. Seismic design category for short period response acceleration S_{DS} [5]

| S_{DS} (g) | Risk Category | |
|--------------------------------|----------------|----------|
| | I or II or III | IV |
| $S_{DS} < 0.167$ | A | A |
| $0.167 \leq S_{DS} \leq 0.133$ | B | C |
| $0.133 \leq S_{DS} \leq 0.50$ | C | D |
| $0.50 \leq S_{DS}$ | D | D |

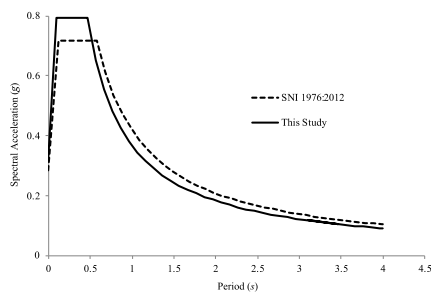
Table 2. Seismic design category for long period response acceleration S_{D1} [5]

| S_{D1} (g) | Risk Category | |
|--------------------------------|----------------|----------|
| | I or II or III | IV |
| $S_{D1} < 0.067$ | A | A |
| $0.067 \leq S_{D1} \leq 0.133$ | B | C |
| $0.133 \leq S_{D1} \leq 0.20$ | C | D |
| $0.20 \leq S_{D1}$ | D | D |

235

236 4.3 Response Spectrum Curve

237 The spectral acceleration parameters previously obtained in Sub Section 4.1 are
 238 described using a response spectrum, which is important for building design as
 239 presented in Fig. 7 and Fig. 8, intended for medium soil (SD) and soft soil (SE)
 240 respectively. For comparison, the dashed line also shows the spectral
 241 acceleration graph based on the current earthquake code SNI 1726:2012. In
 242 general, the acceleration in this work is found relatively greater than that of in
 243 the current code for periods of less than 0.462 s for D site class (SD), and for
 244 periods less than 0.830 s in E site class (SE). The maximum acceleration in SD
 245 is 0.795 g in the period of 0.092-0.462 s. Meanwhile, in site class E, the
 246 maximum spectrum acceleration value is 0.686 g in the period of 0.1166-0.830
 247 s. Also, it can be seen that over this period, medium soils amplify the spectral
 248 acceleration response more than soft soils.



249

Figure 7 Response spectrum for SD

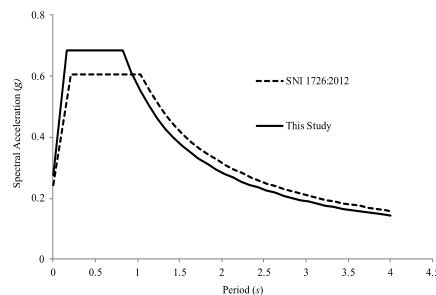


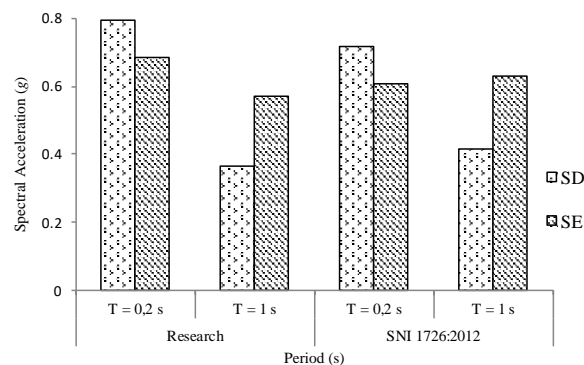
Figure 8 Response spectrum for SE

250

251 However, the soft soils generate the long period response more than the medium
 252 soils. For a period of $T = 1$ s, the spectral acceleration for medium soil is 0.367
 253 g and for soft soil 0.569 g. This trend is consistent with that found in existing
 254 building seismic standards where the medium soil spectrum has an acceleration
 255 of 0.386 g and soft soils of 0.606 g each in the long period. During this period,

256 SNI 1726:2012 shows a slightly higher acceleration than the results of this
257 study.

258 Primarily, a similar shape of the response spectrum curve is seen between the
259 results of this study and the current code. The trend is similar when the medium
260 soil (SD) has higher spectral acceleration than the soft soil (SE) in the short
261 period, but the effect of soft soil higher on spectral acceleration is seen over a
262 longer period as shown in Fig. 9. Such findings are also reported by Dhakal et
263 al. in [26]. During the calculation of seismic loads, the response spectrum is
264 very important. Short period spectral acceleration values are used for an
265 equivalent static analysis to calculate the seismic response factor C_s . Therefore,
266 the effects of the 2018 Lombok earthquake, which produces higher spectral
267 accelerations in a short period of time, may increase the safety of structural
268 designs and improve seismic resistance.



269

270 **Figure 9** Spectral acceleration at T = 0.2 s and T = 1 s for different soil types

271 4.4 Seismic Response Coefficient, C_s

272 Using the procedure for determining seismic response factors (C_s) specified by
273 SNI 1726:2012 in [7], Table 3 shows the values for C_s , maximum C_s , and
274 minimum C_s . The C_s value is calculated under several conditions: risk category
275 = 2, importance factor = 1, response modification factor = 8, building height

276 from base = 20 meters. Coefficients are implemented for both SD and SE types
 277 of site classes. The coefficient calculated based on the current code's spectral
 278 acceleration is also displayed as a comparison.

279

Table 3. Seismic Response Coefficient, C_S

| Seismic Parameter | Site Class D | | Site Class E | |
|----------------------|--------------|-------|--------------|-------|
| | SNI | This | SNI | This |
| | 1726:2012 | Study | 1726:2012 | Study |
| S_{DS} (g) | 0.717 | 0.795 | 0.606 | 0.686 |
| S_{D1} (g) | 0.418 | 0.367 | 0.631 | 0.569 |
| C_S | 0.090 | 0.099 | 0.076 | 0.086 |
| C_S -maximum | 0.766 | 0.673 | 1.156 | 1.044 |
| C_S -minimum | 0.032 | 0.035 | 0.027 | 0.030 |

280

281 All C_S values are between the minimum and maximum C_S values. In general,
 282 the C_S of site class D is higher than the C_S of site class E. This is because C_S
 283 depends on the value of the short period spectral acceleration, S_{DS} . As can be
 284 seen from Table 3, the S_{DS} for site class D is higher than the S_{DS} for site class E.
 285 Therefore, C_S increases in site class D. Conversely, the maximum C_S value for
 286 site class E is greater than the maximum value for site class D because of the
 287 large spectral acceleration value of S_{D1} at $T = 1$ s. The maximum value of C_S
 288 depends on the value of S_{D1} .

289 In this study, both sites have higher C_S results compared to SNI 1976: 2012.
 290 After a strong Lombok earthquake, the seismic coefficient C_S increases by
 291 10.782% when compared to C_S calculated using the current code. The effect is
 292 more severe in soft soil areas, which is an increase of 13.168%. The higher the
 293 C_S , the greater the seismic load on the building structure. It is recommended that
 294 current seismic regulations be revised to consider the effects of the last strong
 295 earthquake, as this will have a significant effect on the increase in seismic loads
 296 experienced by the structure. Changes include enhancements to existing
 297 building structures. Therefore, new or old buildings may be more resistant to
 298 future earthquakes. A similar recommendation has also been delivered by

299 **Ramdani, Setiani, and Setiawati in [27] that studies on the Lombok earthquake**
300 **could support a robust mitigation system for the area.**

301 **5 Conclusion**

302 This study describes the parameters of Mataram's seismic building design by
303 considering the effects of the 2018 earthquake in Lombok.

304 • Short period spectral acceleration, S_s increased 18.323% compared to the
305 values listed in SNI 1976:2012. However, the value of the spectral acceleration
306 in for the period $T = 1$ s, S_1 , is smaller than the value described in the existing
307 earthquake code.

308 • Higher design spectral acceleration values shown in this study do not change
309 the design of the Mataram earthquake category

310 • According to response spectrum curve, overall, the acceleration value in this
311 study is found relatively greater than the that of the existing code for periods of
312 less than 0.462 s for site class D, and in periods less than 0.830 s in site class E.

313 The maximum acceleration in site class D from the results of the study is 0.795
314 g in the period of 0.092 to 0.462 s. For site class E, the maximum spectrum
315 acceleration value is 0.686 g in the period of 0.1166 to 0.830 s.

316 • **Soft soils react longer than medium soils. For the time period of $T = 1$ s, the**
317 **spectral acceleration of medium soil is 0.346 g and soft soil produces 0.553 g.**

318 • Basically, a similar shape of the response spectrum curve is seen between the
319 results of this study and recent codes. Medium soil (SD) has higher spectral
320 acceleration than soft soil (SE) in a short period, but the effect of soft soil higher
321 on spectral acceleration is seen over a longer period.

322 • In this study, both site classes D and E have a higher seismic response
323 coefficient compared to SNI 1976:2012. The seismic coefficient C_s after the
324 Lombok strong ground motion increases by 10.782% compared to the C_s
325 calculated using the current code. Soft soil is more prone because the C_s
326 increases 13.168%.

327 • Immediate revisions to current seismic building codes by considering the
328 impact of the last strong earthquake to strengthen the preparation of seismic
329 structures is recommended.

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1 **An Investigation of Building Seismic Design Parameters in**
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3 **Motion**

4 **Ni Nyoman Kencanawati¹, Didi Supriyadi Agustawijaya¹ & Rian Mahendra**
5 **Taruna²**

6 ¹ Department of Civil Engineering, Mataram University, Jl. Majapahit No. 62, 83115
7 Mataram, Indonesia

8 ² Meteorology, Climatology and Geophysics Agency, Jl. Tgh. Ibrahim Khalidi, 83362
9 Lombok Barat, Indonesia
10 Email: nkencanawati@unram.ac.id

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13 **Novelty:** The strong earthquake in Lombok in 2018 caused an increase in
14 spectral acceleration compared to what is stated in Indonesia's current earthquake
15 code. As a result, changes affect building design parameters. This paper showed
16 that the seismic response factor of the building increased by 10.782% and
17 13.168% on medium and soft soil, respectively, compared to that of the current
18 code. It also recommends that seismic codes need to be improved to provide
19 better preparedness for future seismic risk reduction.

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Highlight:

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- Spectral acceleration using the Lombok Earthquake 2018 is analyzed,
- The spectral acceleration is greater than the existing seismic code acceleration,
- The seismic response coefficient is higher than the existing seismic codes,
- Existing seismic building standards need to be improved.

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Abstract. Mataram is the capital of West Nusa Tenggara. West Nusa Tenggara is made up of two islands, Lombok and Sumbawa. The 2018 earthquake on Lombok has undoubtedly affected spectral acceleration. This is an important factor to be addressed in structural design. Short period spectral acceleration, S_S increases 18.323% compared to the value listed in the seismic code SNI 1976:2012 corresponding to the 2500 return period. However, even if the S_S value increases, the design category of the building does not change and remains in the D category. In general, the acceleration value in this study is found relatively greater than that of the existing code for periods of less than 0.462 s for site class D, and in periods of less than 0.830 s in site class E. In addition, the seismic response coefficient, C_s , for medium soil, it increases by 10.782% compared to the C_s calculated using of the current code. This effect is more severe in soft soil areas where the increase reaches 13.168%. Improving existing codes with seismic design parameters for new buildings affected by the ground motion of recent strong earthquakes will lead to more preparedness and will be an important part of local disaster risk reduction.

41 **Keywords:** *Lombok earthquake 2018; spectral acceleration; seismic design*
42 *parameters, seismic code*

43 **1 Introduction**

44 The West Nusa Tenggara region is an area of high seismic activity, surrounded
45 by two active seismic sources. In the south is the subduction zone of the Indo-
46 Australia Sea Plate, and in the north is the back-arc thrust zone. According to
47 the National Institutes of Meteorology, Climatology, and Geophysics
48 (Indonesian: Badan Meteorologi, Klimatologi, dan Geofisika (BMKG)), a
49 magnitude 6.2 magnitude earthquake on June 9, 2016, occurred in Mataram and
50 Central Lombok and caused some damages. Later, in 2017, several quakes were
51 hit at the scale of II-III MMI in Mataram City, as written by Taruna et al. in [1].
52 In addition, officials reported that there were 3699 earthquake events in 2018
53 and 215 events were felt. One of a series of Lombok earthquakes on August 5,
54 2018, with a magnitude of 7.0, caused severe damage to a number of buildings
55 and houses, even some collapsed in Lombok area, including the city of
56 Mataram, as announced by BMKG in [2] and published by Pomonis in [3] and
57 Asmirza in [4].

58 In the past, some countries have changed their seismic codes after large
59 earthquakes that caused various damage to structures and buildings. As studied
60 by Okamura in [5] and Karakostas et al. in [6], the seismic code has been
61 improved with a new response spectrum affected by recent ground acceleration.
62 Similarly, Indonesia has the current code for seismic structures, namely SNI
63 1726: 2012 in [7]. The ground motion is calculated with a 2% probability of
64 being exceeded within 50 years. The return period of the spectral acceleration is
65 2500 years. It replaces SNI 1726:2002 in [8]. SNI 1726:2002 provides spectral
66 acceleration by dividing all areas of Indonesia into six seismic zones. The
67 current Seismic Building Code has been improved by providing spectrally
68 accelerated design values at each coordinate point in Indonesia. Seismic

69 acceleration maps are also attached for spectral accelerations at $T = 0$ s (PGA),
70 $T = 0.2$ s (short period), and $T = 1$ s (long period).

71 **The previous seismic design code, SNI 1726:2002, has been reviewed by**
72 **Sengara et al. [9].** In addition, compared to the previous seismic code SNI
73 1726:2002 presented by Arfiadi and Satyarno in [10], some of the Indonesian
74 short period design spectral acceleration, S_{DS} , have a significant increase in
75 current seismic code SNI 1726:2012. Significant increases in S_{DS} are evident in
76 some areas, such as Aceh, Palu, Yogyakarta, and Padang, which were affected
77 by major earthquakes during the time when the previous code was applied.
78 Therefore, the values have been modified in the current code. In Palu, S_{DS} has
79 the largest increase, with 116.7%, 85.7% and 41.2% in hard, medium, and soft
80 soils, respectively. This region was hit by a magnitude 7.7 earthquake in 2008,
81 and the 2012 seismic code changed the spectral acceleration. In the other prone
82 areas mentioned above, the S_{DS} of three types of soil has risen from 10% to
83 80%. Conversely, Lombok did not show significant seismic activity during that
84 period. Therefore, the 2012 seismic code shows little change in acceleration.

85 To obtain a new spectral acceleration that includes the site amplification factor,
86 strong ground motions after the earthquake must be considered. This is
87 compared to the existing spectral acceleration provided by the existing code to
88 make sure there is a sufficient design to face strong earthquakes that may occur
89 in the future, as reported by Panzera et al. in [11] and Mase, Likitlersuang, and
90 Tobita in [12]. Furthermore, the evaluation of seismic codes after earthquakes
91 has been carried out in some countries. The earthquake code has been updated
92 to consider the recent ground acceleration due to the earthquake. In addition to
93 the response spectrum, details of the structural design have been improved
94 further as given by Okamura in [5], Karakostas et al. in [6], Sezen et al. in [13],

95 Ergün, Kiraç, and Bacsaran in [14], Mosleh et al. in [15] and Baros and Santa-
96 Maria in [16].

97 The analysis describes the seismic hazard in Mataram city using seismic data up
98 to 2017 with a 2% probability of being exceeded in 50 years (return period of
99 2500 years). The short period of bedrock acceleration S_S ($T = 0.2$ s) and the
100 long period of bedrock acceleration S_1 ($T = 1$ s) were reported to be in the range
101 of 0.37-0.45 g ($g = 9.81\text{m/s}^2$) and 0.16-0.18 g , respectively. Furthermore, the
102 values of S_S and S_1 in the northern region of Mataram are higher than those in
103 the southern region of Mataram. This is due to the superiority of the Back Arc
104 Thrust activity in northern Lombok as given by Taruna in [1].

105 The 2018 earthquake on Lombok is an important consideration in spectral
106 acceleration. This is an important factor to be addressed in structural design.
107 Improving the calculation of parameters will lead to the reproduction of the
108 structural design under seismic loading, which is part of disaster risk reduction.
109 It could potentially save millions of people and reduce major risks in the region
110 in the future. Therefore, a new spectral acceleration needs to be approached
111 using the recent 2018 seismic data, which applies to some seismic parameters
112 that will help better seismic structures.

113 **2 Related Research and Theory**

114 According to Agustawijaya, Sulistyono, and Elhuda in [17], Lombok is
115 classified as moderate to high seismic activity. Before the strong earthquake of
116 2018, this study states that the South Subduction Megathrust and the North
117 Back-Arc Thrust have established the tectonic pattern of Lombok Island as an
118 effect of compression between the Australian continental plate and Eurasia.
119 Then in 2018, a series of earthquakes occurred in North Lombok which was
120 triggered by Flores back arc thrust. The ground motion initially began on July 28

121 with an M_w 6.4 earthquake in the northern part of Lombok. Aftershocks with
122 $M_w < 5$ followed the first earthquake a few hours later. On August 5, a larger
123 shock of M_w 7.0 occurred. Then, in the following two weeks, an M_w 6.9
124 earthquake hit the island on August 19, 2018. The sequences of Lombok ground
125 motions have been studied in detail by some researchers in [18], [19].

126 As reported by Marjiyono in [20], in general, the plains of Mataram City are
127 dominated by alluvial deposits with sandy materials, either product of the
128 eastern river process or marine products of the West Side. The alluvium fills an
129 ancient form in the form of a basin in the western part of Mataram. Physically,
130 alluvial sediments are soft and are indicated by low shear wave velocity values.
131 This condition is potentially for areas that experience such wave amplification
132 during an earthquake [21]. In addition, the average measurement of shear wave
133 velocity v_s shows the value range of 135-201 m/s in Mataram City. This value is
134 included in site class D (SD) and site class E (SE) of the current building
135 seismic code.

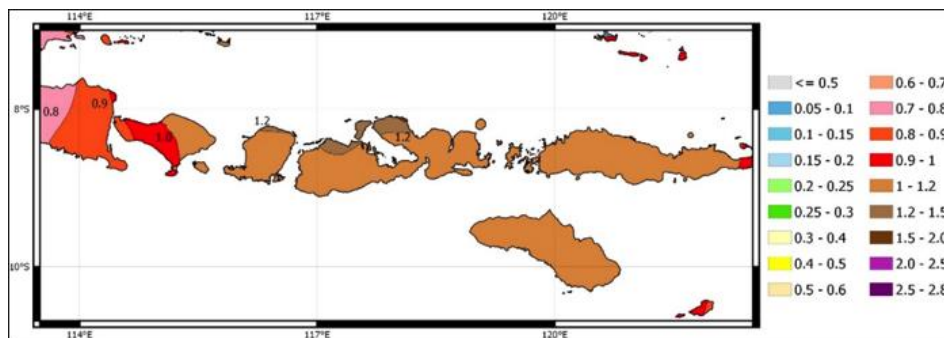
136 S_s and S_1 must be determined at $T = 0.2$ s and $T = 1$ s, respectively, provided in
137 the ground motion map of the SNI 1726:2012 code, and may exceed 2% in 50
138 years. By multiplying the S_s and S_1 values by the amplification factor from each
139 site class, the short-period, S_{MS} , and long-term S_{M1} surface maximum ground
140 acceleration can be calculated directly [22], [23]. The amplification factor F_a is
141 related to the acceleration of the short-period S_s , while the amplification factor
142 associated with S_1 is F_v . Furthermore, S_{MS} and S_{M1} values are used to calculate
143 design spectral acceleration parameters for short period, S_{DS} and long period
144 S_{D1} , as described by in SNI 1726:2012 in [7].

145 **3 Method**

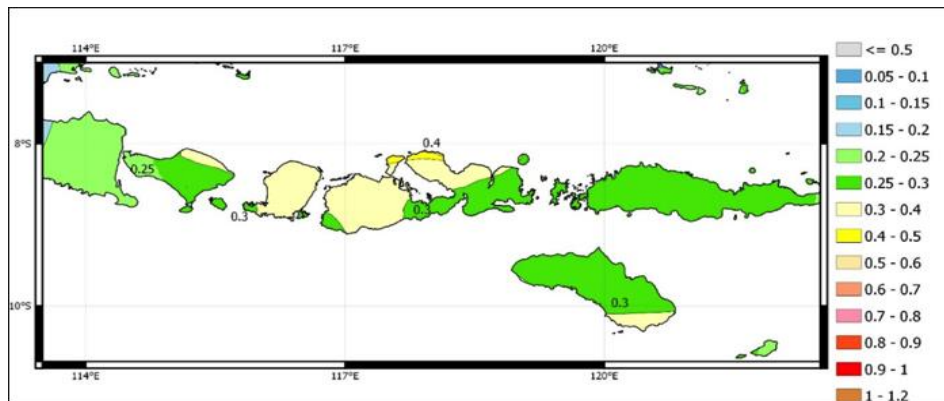
146 **3.1 Ground Acceleration Data**

147 Ground acceleration data is based on previous work studied by Taruna,
 148 Agustawijaya, and Kencanawati in [24]. Earthquake data was obtained from
 149 Engdahl ISC (EHB), USGS, and BMKG in 1922-2018. The data was taken at
 150 coordinates of the latitude of 7° - 12° and longitude of 113.5° - 122.5° or about
 151 300 km from Mataram City with magnitude, $M_w \geq 4.5$. This magnitude is
 152 assumed to be the standard for earthquakes related to the risk of seismic
 153 disasters. In this study, the values of peak ground motion in the bedrock soil
 154 layer from the previous study are used. Ground motion or maximum
 155 acceleration will be adopted as the parameters used in this study. These
 156 parameters are S_S and S_1 which are related to the technical design of earthquake-
 157 resistant structures, as shown in Figs. 1-2.

158



159 **Figure 1** Spectral Acceleration at $T = 0.2$ s in bedrock with a probability exceeding 2%
 160 in 50 years for Bali-West Nusa Tenggara region, Taruna in [24]



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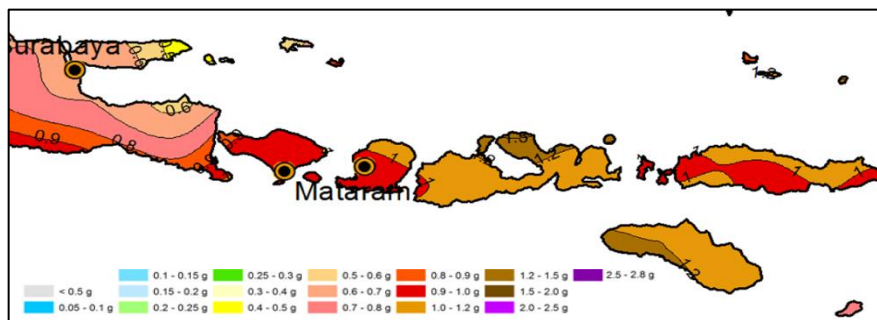
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Figure 2 Spectral Acceleration at $T = 1$ s in bedrock with a probability exceeding 2% in 50 years for Bali-West Nusa Tenggara region, Taruna in [24]

164

As shown in Fig. 1, in most Lombok regions, S_S values range from 1-1.2 g, and in North Lombok, the values are over 1.2 g. S_S values tend to be larger than the 0.9-1.2 g values for Lombok, calculated in SNI 1726:2012 (Fig. 3). This could be caused by the large earthquake data used in previous studies, especially the increase in the 2018 Lombok earthquake series. On the other hand, the maximum acceleration of S_1 is 0.25 to 0.4 g. The S_1 values in the Lombok region used in this study are lower than those of SNI 1726:2012 (Fig. 4). In SNI 1726:2012, Lombok's S_1 values range from 0.3 to 0.5 g, with the maximum seen in the north.

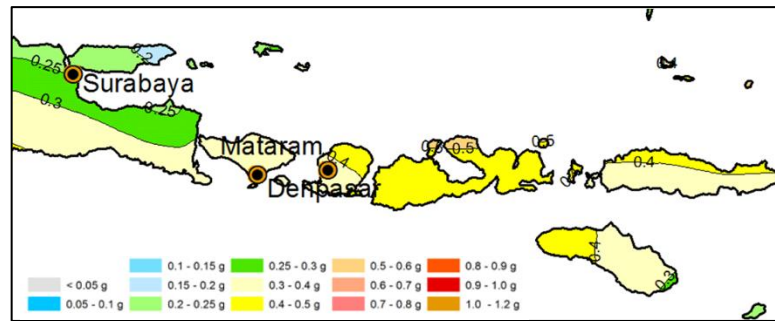
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Figure 3 Spectral Acceleration at $T = 0.2$ s in bedrock with a probability exceeding 2% in 50 years for the Bali-West Nusa Tenggara region, SNI 1726:2012 in [7]



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Figure 4 Spectral Acceleration at $T = 1$ s in bedrock with a probability exceeding 2% in 50 years for the Bali-West Nusa Tenggara region, SNI 1726:2012 in [7]

179 3.2 Equivalent Lateral Load Factor

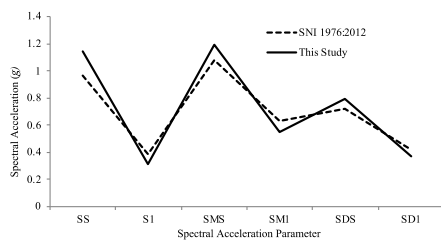
180 The dynamic properties of seismic loads are simplified to horizontal forces with
 181 an equivalent lateral load procedure. For the analysis, the seismic response
 182 coefficient C_s is determined. SNI 1726:2012 provides instructions for obtaining
 183 C_s . It depends on spectral acceleration, S_{DS} and S_{D1} values and parameters such
 184 as seismic design category, importance factor, structural fundamental period,
 185 response modification factor, etc.

186 4 Result and Discussion

187 4.1 Spectral Acceleration

188 As shown in Figs. 5-6, the strong earthquake in Lombok in 2018 increased the
 189 spectral acceleration S_s of Mataram by 1.143 g. This value represents the
 190 location of Mataram latitude: -8.5606 and longitude: 116.0707. This is an
 191 increase of 18.323% from the value listed in SNI 1976:2012. Approximately the
 192 same increase as the spectral acceleration in Padang City when provided in the
 193 previous seismic code compared to the current code (SNI 1976:2012). This is
 194 because Padang experienced a major earthquake in 2009, the transition period
 195 between the previous code and the current code. The following Indonesian
 196 seismic code assumed that the acceleration of Mataram would be potentially

197 higher due to the 2018 Lombok earthquake, as this study shows. Meanwhile,
 198 Sharma in [25] reported that after the Nepal earthquake (M_w 7.9) ground
 199 motion, existing spectral accelerations were still applicable to seismic structural
 200 engineering design.



201

Figure 5 Spectral acceleration parameters in medium soil

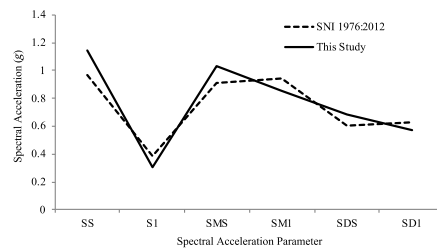


Figure 6 Spectral acceleration parameters in soft soil

202 Figure 5-6 also shows the spectral acceleration of the maximum considered and
 203 design basis earthquakes on the surface at $T = 0.2$ s (S_{MS} and S_{DS}) and $T = 1$ s
 204 (S_{M1} and S_{D1}) on medium soil (Fig. 5) whereas the parameters for soft soil is
 205 illustrated in Fig. 6. The acceleration of the surface is calculated for site class D
 206 (SD) and site class E (SE) because Mataram city is made up of medium and soft
 207 soils as given by Marjiyono in [20]. The spectral acceleration provided by SNI
 208 1726: 2012 is also shown for comparison.

209 Contrary to the acceleration of $T = 0.2$ s, the acceleration of $T = 1$ s used in this
 210 study is smaller than that of SNI 1726:2012 because the constant attenuation
 211 equation is not the same between S_s and S_1 . Furthermore, theoretically, S_1 is a
 212 long period spectrum affected by far-field earthquakes. On the other hand, this
 213 study is more dominant near earthquakes.

214 4.2 Building Seismic Design Category

215 Considering the ground motion of recent earthquakes, the S_{DS} and S_{D1} values for
 216 Mataram are 0.795 g and 0.367 g for medium soil and 0.686 g and 0.569 g for

217 soft soil, respectively. According to SNI 1976:2012, buildings at sites with an
 218 S_{DS} greater than 0.5 g are designed as D categories for all risk categories I-IV
 219 (shown in bold in Table 1). Similarly, for S_{D1} values, as shown in Table 2,
 220 Mataram has values greater than 0.2 g in both medium and soft soils. Therefore,
 221 it is included in the D seismic design category (shown in bold in Table 2). S_{DS}
 222 values exceed 0.5 g and S_{D1} values exceed 0.2 g. This is similar to the value in
 223 the current code. Thus, even though the results of the study's spectral
 224 acceleration appear larger than those present in the current seismic code, there is
 225 no change in the seismic design category between the current seismic code and
 226 the results of this study.

227 The D-design seismic category is intended for structures built in the sites which
 228 to be potential for severe and damaging earthquakes, but not located close to
 229 major faults. As given by Giouncu and Mazolani in [22] dan Duggal in [23],
 230 structures on poor soils generally fall into the D class for seismic design.
 231 According to Sharma et al. in [25] as mentioned above, there is no change in the
 232 spectral acceleration between the existing code and the spectral acceleration
 233 after the Nepal earthquake, however, it is recommended to implement the
 234 existing code to develop mitigation strategies and structures.

Table 1. Seismic design category for short period response acceleration S_{DS} [5]

| S_{DS} (g) | Risk Category | |
|--------------------------------------|----------------|----------|
| | I or II or III | IV |
| $S_{DS} < 0,167$ | A | A |
| $0.167 \leq S_{DS} \leq 0.133$ | B | C |
| $0.133 \leq S_{DS} \leq 0.50$ | C | D |
| $0.50 \leq S_{DS}$ | D | D |

Table 2. Seismic design category for long period response acceleration S_{D1} [5]

| S_{D1} (g) | Risk Category | |
|--------------------------------------|----------------|----------|
| | I or II or III | IV |
| $S_{D1} < 0,067$ | A | A |
| $0.067 \leq S_{D1} \leq 0.133$ | B | C |
| $0.133 \leq S_{D1} \leq 0.20$ | C | D |
| $0.20 \leq S_{D1}$ | D | D |

235

236 **4.3 Response Spectrum Curve**

237 The spectral acceleration parameters previously obtained in Sub Section 4.1 are
 238 described using a response spectrum, which is important for building design as
 239 presented in Fig. 7 and Fig. 8, intended for medium soil (SD) and soft soil (SE)
 240 respectively. For comparison, the dashed line also shows the spectral
 241 acceleration graph based on the current earthquake code SNI 1726:2012. In
 242 general, the acceleration in this work is found relatively greater than that of in
 243 the current code for periods of less than 0.462 s for D site class (SD), and for
 244 periods less than 0.830 s in E site class (SE). The maximum acceleration in SD
 245 is 0.795 g in the period of 0.092-0.462 s. Meanwhile, in site class E, the
 246 maximum spectrum acceleration value is 0.686 g in the period of 0.1166-0.830
 247 s. Also, it can be seen that over this period, medium soils amplify the spectral
 248 acceleration response more than soft soils.

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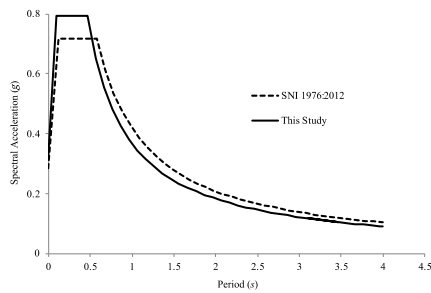


Figure 7 Response spectrum for SD

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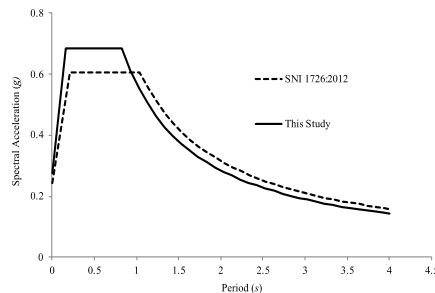
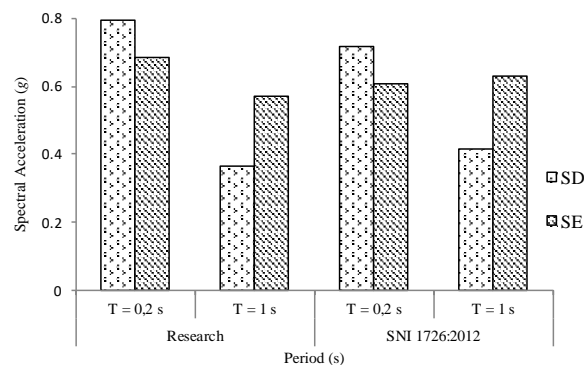


Figure 8 Response spectrum for SE

251 However, the soft soils generate the long period response more than the medium
 252 soils. For a period of $T = 1$ s, the spectral acceleration for medium soil is 0.367
 253 g and for soft soil 0.569 g. This trend is consistent with that found in existing
 254 building seismic standards where the medium soil spectrum has an acceleration
 255 of 0.386 g and soft soils of 0.606 g each in the long period. During this period,

256 SNI 1726:2012 shows a slightly higher acceleration than the results of this
257 study.

258 Primarily, a similar shape of the response spectrum curve is seen between the
259 results of this study and the current code. The trend is similar when the medium
260 soil (SD) has higher spectral acceleration than the soft soil (SE) in the short
261 period, but the effect of soft soil higher on spectral acceleration is seen over a
262 longer period as shown in Fig. 9. Such findings are also reported by Dhakal et
263 al. in [26]. During the calculation of seismic loads, the response spectrum is
264 very important. Short period spectral acceleration values are used for an
265 equivalent static analysis to calculate the seismic response factor C_S . Therefore,
266 the effects of the 2018 Lombok earthquake, which produces higher spectral
267 accelerations in a short period of time, may increase the safety of structural
268 designs and improve seismic resistance.



269

270 **Figure 9** Spectral acceleration at T = 0.2 s and T = 1 s for different soil types

271 4.4 Seismic Response Coefficient, C_S

272 Using the procedure for determining seismic response factors (C_S) specified by
273 SNI 1726:2012 in [7], Table 3 shows the values for C_S , maximum C_S , and
274 minimum C_S . The C_S value is calculated under several conditions: risk category
275 = 2, importance factor = 1, response modification factor = 8, building height

276 from base = 20 meters. Coefficients are implemented for both SD and SE types
 277 of site classes. The coefficient calculated based on the current code's spectral
 278 acceleration is also displayed as a comparison.

279

Table 3. Seismic Response Coefficient, C_S

| Seismic Parameter | Site Class D | | Site Class E | |
|----------------------|--------------|-------|--------------|-------|
| | SNI | This | SNI | This |
| | 1726:2012 | Study | 1726:2012 | Study |
| S_{DS} (g) | 0.717 | 0.795 | 0.606 | 0.686 |
| S_{D1} (g) | 0.418 | 0.367 | 0.631 | 0.569 |
| C_S | 0.090 | 0.099 | 0.076 | 0.086 |
| C_S -maximum | 0.766 | 0.673 | 1.156 | 1.044 |
| C_S -minimum | 0.032 | 0.035 | 0.027 | 0.030 |

280

281 All C_S values are between the minimum and maximum C_S values. In general,
 282 the C_S of site class D is higher than the C_S of site class E. This is because C_S
 283 depends on the value of the short period spectral acceleration, S_{DS} . As can be
 284 seen from Table 3, the S_{DS} for site class D is higher than the S_{DS} for site class E.
 285 Therefore, C_S increases in site class D. Conversely, the maximum C_S value for
 286 site class E is greater than the maximum value for site class D because of the
 287 large spectral acceleration value of S_{D1} at $T = 1$ s. The maximum value of C_S
 288 depends on the value of S_{D1} .

289 In this study, both sites have higher C_S results compared to SNI 1976: 2012.
 290 After a strong Lombok earthquake, the seismic coefficient C_S increases by
 291 10.782% when compared to C_S calculated using the current code. The effect is
 292 more severe in soft soil areas, which is an increase of 13.168%. The higher the
 293 C_S , the greater the seismic load on the building structure. It is recommended that
 294 current seismic regulations be revised to consider the effects of the last strong
 295 earthquake, as this will have a significant effect on the increase in seismic loads
 296 experienced by the structure. Changes include enhancements to existing
 297 building structures. Therefore, new or old buildings may be more resistant to
 298 future earthquakes. A similar recommendation has also been delivered by

299 Ramdani, Setiani, and Setiawati in [27] that studies on the Lombok earthquake
300 could support a robust mitigation system for the area. Other works related to the
301 Lombok post-earthquake evidence showed that several damages on concrete
302 structures and steel structures existed as evaluated by Salim et al. in [28].
303 Further improvements have been recommended to the structures by considering
304 the basic requirement for earthquake resistance structures as given by Siswanto
305 and Salim in [29]. In addition, such a comprehensive structural design based on
306 seismic risk has been introduced by Mangkoesobroto, Prayoga, and Parithusta
307 in [30] and Sidi in [31]. It is suggested that the presented works shall be
308 considered for the next seismic code to be a better structural response against
309 future earthquakes.

310 5 Conclusion

311 This study describes the parameters of Mataram's seismic building design by
312 considering the effects of the 2018 earthquake in Lombok.

- 313 • Short period spectral acceleration, S_s increased 18.323% compared to the
314 values listed in SNI 1976:2012. However, the value of the spectral acceleration
315 in for the period $T = 1$ s, S_1 , is smaller than the value described in the existing
316 earthquake code.
- 317 • Higher design spectral acceleration values shown in this study do not change
318 the design of the Mataram earthquake category
- 319 • According to response spectrum curve, overall, the acceleration value in this
320 study is found relatively greater than the that of the existing code for periods of
321 less than 0.462 s for site class D, and in periods less than 0.830 s in site class E.
322 The maximum acceleration in site class D from the results of the study is 0.795
323 g in the period of 0.092 to 0.462 s. For site class E, the maximum spectrum
324 acceleration value is 0.686 g in the period of 0.1166 to 0.830 s.
- 325 • Soft soils react longer than medium soils. For the time period of $T = 1$ s, the
326 spectral acceleration of medium soil is 0.346 g and soft soil produces 0.553 g.

- 327 • Basically, a similar shape of the response spectrum curve is seen between the
328 results of this study and recent codes. Medium soil (SD) has higher spectral
329 acceleration than soft soil (SE) in a short period, but the effect of soft soil higher
330 on spectral acceleration is seen over a longer period.
- 331 • In this study, both site classes D and E have a higher seismic response
332 coefficient compared to SNI 1976:2012. The seismic coefficient C_s after the
333 Lombok strong ground motion increases by 10.782% compared to the C_s
334 calculated using the current code. Soft soil is more prone because the C_s
335 increases 13.168%.
- 336 • Immediate revisions to current seismic building codes by considering the
337 impact of the last strong earthquake to strengthen the preparation of seismic
338 structures is recommended.

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Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

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Journal of Engineering and Technological Sciences
Institut Teknologi Bandung
jets@lppm.itb.ac.id



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Journal of Engineering and Technological Sciences
Institut Teknologi Bandung
jets@lppm.itb.ac.id



Ni Nyoman Kencanawati <nkencanawati@unram.ac.id>

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The West Nusa Tenggara region is an area of high seismic activity surrounded by two active seismic sources. In the south is the subduction zone of the Indo-Australian Plate (IAP) and in the north is the subduction zone of the Indo-Australian Plate (IAP).
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