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Lombok, Indonesia, September 14<sup>th</sup>, 2022



**Dr. Nur Kaliwantoro, ST., MT.**  
General Chairman of the 1<sup>st</sup> MIMSE 2022





# Peer-Review Statements

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All of the articles in this proceedings volume have been presented at the 1<sup>st</sup> MIMSE 2022 on September 14, 2022 in Mataram, Indonesia. These articles have been peer reviewed by the members of the Scientific Committee and approved by the Editor-in-Chief, who affirms that this document is a truthful description of the conference's review process.

## 1 Review Procedure

The reviews were single-blind. Each submission was examined by at least 1 reviewer independently.

The conference submission management system was *EasyChair*.

The submissions were first screened for generic quality and suitability. After the initial screening, they were sent for peer review by matching each paper's topic with the reviewers' expertise, taking into account any competing interests. A paper could only be considered for acceptance if it had received favourable recommendations from at least one reviewer.

Authors of a rejected submission were given the opportunity to revise and resubmit after addressing the reviewers' comments. The acceptance or rejection of a revised manuscript was final.

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Reviewers were instructed to assess the quality of submissions solely based on the academic merit of their content along the following dimensions:

1. Pertinence of the article's content to the scope and themes of the conference;
2. Clear demonstration of originality, novelty, and timeliness of the research;
3. Soundness of the methods, analyses, and results;
4. Adherence to the ethical standards and codes of conduct relevant to the research field;
5. Clarity, cohesion, and accuracy in language and other modes of expression, including figures and tables.

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B. Anshari—Editor-in-Chief the 1<sup>st</sup> MIMSE 2022\_CA.

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Number of articles sent for peer review	25
Number of accepted articles	21
Acceptance rate	84%
Number of reviewers	16

### 4 Competing Interests

Neither the Editor-in-Chief nor any member of the Scientific Committee declares any competing interest.

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# Proceedings of the First Mandalika International Multi-Conference on Science and Engineering 2022, MIMSE 2022 (Civil and Architecture)

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## PREFACE

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Conference name: Proceedings of the First Mandalika International Multi-Conference on Science and Engineering 2022, MIMSE 2022 (Civil and Architecture)

Date: 14-15 September 2022

Location: Mataram, Indonesia (Hybrid)

Website: <https://mimse.unram.ac.id/>

The 1st Mandalika International Multi-Conference on Science and Engineering 2022– Track Civil Engineering and Architecture is designed as an environment for researchers to discuss the current state of the science and technology in industry, university and companies. The conference is held hybrid at the Hotel Lombok Raya, Lombok, NTB, Indonesia, on September 14, 2022. This conference is organized by the Faculty of Engineering, University of Mataram, West Nusa Tenggara, in collaboration with the University of Esa Unggul Jakarta. The MIMSE 2022 has theme of “smart and green technology for a better life”.

**The articles of the 1st MIMSE 2022–Civil Engineering and Architecture Track come from seven countries all over the world.** After peer-review, 21 papers were selected to be published in the Atlantis Publisher. Several excellent keynote and

invited speakers presented state-of-the art findings in the science and engineering. This conference is the result of the hard work, support and dedication of a number of parties. We wish to thank all the committee members who together make the conference possible. We also want to thank our partners for the funding of the conference.

Yours sincerely,

Nur Kaliwantoro

1st MIMSE 2022 Chair

October 2022

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# Assessment Factor of Strength Development for Normal, High Strength, and Lightweight Concretes

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and Ricko Fachri Afriandi

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**Abstract.** Development in the construction sector continues to increase. The most common building material nowadays is concrete. Although normal concrete is often is being used, at this time high strength concrete and lightweight concrete have also been widely used in construction. In the codes for concrete materials, either SNI 03-6805-2002 or PBI NI-2 1971, it is stated about the assessment factor of compressive strength development for normal concrete according to the age of the concrete. However, these codes have not accommodated the assessment factor for high-strength and lightweight concrete. An experimental approach was used to determine the assessment factor and is discussed in this paper. The specimens were cylinders of high strength concrete, normal concrete, and lightweight concrete and tested for compressive strength after curing times of 3, 7, 14, 21, 28 days, 56, and 90 days. According to the experimental result, it presents that the concrete compressive strength increases with the increasing of concrete age. The assessment factor for the development of compressive strength of high-strength concrete shows the highest value, while lightweight concrete provides the lowest factor. The assessment factors of compressive strength development for normal concrete lie in between the values given in PBI NI-2 1971 and SNI 03-6805-2002. Meanwhile, the assessment factors stated in SNI 03-6805-2002 retains the highest value.

**Keywords:** Normal Concrete · High Strength Concrete · Lightweight Concrete · Assessment Factor · Compressive Strength

## 1 Introduction

Predominantly most areas of the world utilize concrete as material construction. The main advantage offered by concrete is that it has high compressive strength compared to other building materials. Normally, concrete is composed by a normal coarse and fine aggregate, cement, and water in a certain ratio to produce a normal or conventional concrete. Normal concrete has a weight of 2200–2500 kg/m<sup>3</sup> [1–3].

Concrete technology has been developing forward and discovering concretes with various modifications, such as lightweight concrete and high-strength concrete [4–6].

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Lightweight concrete is a concrete that has a specific gravity lighter than normal concrete in general. It offers a benefit mainly in reducing the seismic load sustained by the structure. According to ASTM C567 concerning of Standard Test Method for Determining Density of Structural Lightweight Concrete and SNI 2487-2019 concerning of Procedure for Calculation of Concrete Structures for Building and Non-Building, lightweight concrete is determined as a concrete with density between 1140 and 1840 kg/m<sup>3</sup>.

Meanwhile a higher compressive strength is provided by a high strength type concrete compared to a conventional normal concrete; there for e the dimension of the structural element can be reduced. In addition, high strength type concrete is a concrete which owns an advanced mechanical property when related to ordinary normal concrete. A concrete can categorized as a high strength concrete when it presents a compressive strength of between 40–80 MPa. According to the code of Procedure For Planning A Mixture Of High Strength Concrete With Portland Cement and Fly Ash [7], the strength of a concrete must be exceeded a value of 41,4 MPa when it is determined as a high strength concrete. High strength concrete has a greater compressive strength because the mixture uses certain material admixtures [8].

The assessment factor is used to predict the compressive strength of concrete at a certain age. The age of 28 days is used as a standard in determining this factor assessment because after curing time of 28 days, the concrete almost approaches its maximum strength. In terms of the development of compressive strength, based on the code: SNI 03-6805-2002 it is stated that the ratio of the concrete ages to the concrete compressive strength is directly proportional [3, 9–11]. There has been an assessment factor for concrete compressive strength development either in the current code (SNI 03-6805-2002) [12] or the outdated concrete code (PBI NI-2 1971) [13], namely for 3 days, 7 days, 14 days, 21 days, 28 days, 90 days and 365 days. However, the assessment factor for concrete compressive strength development stated in those codes is only for normal concrete, while for high-strength concrete and lightweight concrete does not yet exist.

Therefore, this paper provides a discussion of the assessment factor for concrete compressive strength development according to the outcomes of the experimental approach in the laboratory.

## 2 Methodology

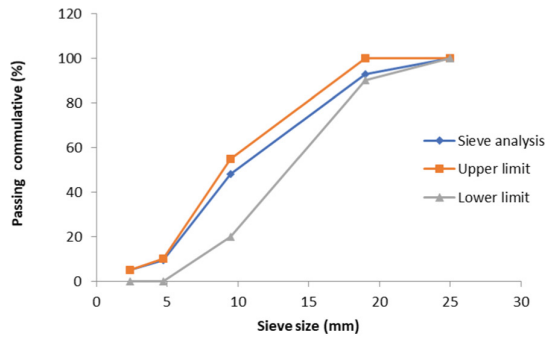
### 2.1 Materials

The main ingredients used in this research were cement, coarse aggregate, fine aggregate, and water. Pozzolan portland cement (PPC) type I manufactured by Tiga Roda was used as cement material. Coarse aggregate in the form of crushed stone with a maximum size of 20 mm was used in normal and high strength concrete, whereas pumice with a maximum size of 20 mm was used as coarse aggregate in lightweight concrete. All types of concrete used fine aggregate of river sand with a maximum diameter of 4,75 mm. The examination of the aggregate was according to the national standard [14]. The aggregate properties are shown in Table 1 and Fig. 1.

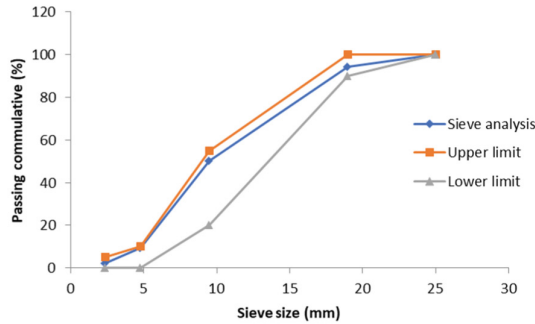


**Table 1.** Properties of aggregates.

Properties	Aggregate type		
	Fine aggregate	Normal	Lightweight
		coarse aggregate	coarse aggregate
Saturated surface dry density (kg/cm <sup>3</sup> )	2588	2660	1200
Fineness modulus	3,4	6,33	6,34
Impurities content (%)	1,85	1,06	1,62
Los Angeles Abrasion resistant (%)	-	26,24	26,98



(a)



(b)

**Fig. 1.** Sieve analysis of (a) normal coarse aggregate and (b) lightweight coarse aggregate.

**2.2 Concrete Mixture Proportion**

The concrete were categorized into three types during the experiment, namely normal concrete, high strength concrete, and lightweight concrete. Normal concrete was designed with a quality of 25 MPa at the age of 28 days and a water-cement ratio of 0,61. High-strength concrete was designed with a quality of 45 MPa and a water-cement

**Table 2.** Concrete Mixture Proportion

Mixture Proportion (kg/m <sup>3</sup> )	Concrete type		
	Lightweight	Normal	High strength
	concrete	concrete	concrete
Water	327,2	167,2	178,5
Cement	414,2	336,1	683,3
Sand	414,3	964,6	569,1
Crushed stone	-	923,4	923,4
Pumice	388,1	-	-
Silica fume	-	-	68,3
Superplasticizer	-	-	4,1

ratio of 0,3. Both normal concrete and high strength concrete utilized coarse aggregate in the form of crushed stone. Due to the low water-cement ratio, the high-strength concrete required superplasticizer to increase its workability. Likewise, to achieve a high quality, the high-strength concrete mixture was added of silica fume as much as 10 percent of the cement weight. Furthermore, lightweight concrete was designed to achieve a quality of 17 MPa and a water-cement ratio of 0,58. Pumice was utilized as coarse aggregate in this lightweight concrete mix. The mixture proportion of each concrete type is explained in Table 2 [3].

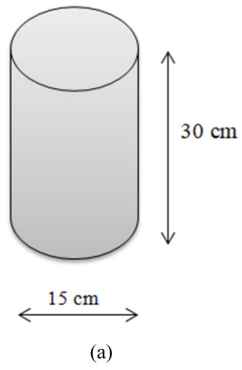
### 2.3 Specimens and Testing

The specimens were standard concrete cylinder with a diameter of 150 mm and a height of 300 mm. Testing was carried out by applying a compressive load to the test object until failure [15]. Tests were carried out at the specimens ages were 3, 7, 14, 21, 28, 56, and 90 days. The sketch of the specimen and the method of testing can be seen in Fig. 2.

## 3 Results and Discussion

### 3.1 Fresh Concrete Properties

In general, the slump value is obtained between 7,5 cm–12 cm for all types of concrete which means that a mixture of lightweight concrete, normal concrete, and high strength concrete are workable or being processed well. Lightweight and normal concrete reach the slump value of 7,5 cm and 8 cm respectively. Unlike high strength concrete, due to the lower water-cement ratio, a superplasticizer is required in the mixture so that the mixture reached a slump value of 12 cm. The superplasticizer used is Viscocrete 1003 with an amount of 0.6% by weight of cement. Table 3 indicates the average of slump values of each concrete type.



**Fig. 2.** The sketch of the specimen (a), the specimens (b), and the method of testing (c).

**Table 3.** Concrete Slump Value

Type	Average of slump (mm)
Lightweight concrete	7,5
Normal concrete	8
High strength concrete	12

### 3.2 Properties of the Concrete on 28 Days Curing Time

Post curing time of 28 days, lightweight concrete reaches a density of  $1500 \text{ kg/m}^3$ . This value corresponds to those of structural lightweight concrete required by SNI 2847-2019, which is  $1140\text{--}1840 \text{ kg/m}^3$ . Similarly, the compressive strength of 28 days, which is 18 MPa, meets the requirements as lightweight concrete for structural applications.

Normal concrete has a volume weight of  $2230 \text{ kg/m}^3$  and a compressive strength of 32,7 MPa at the age of 28 days. Both in terms of density and compressive strength, this concrete meets the requirements to be applied as a structural element. Likewise, high-strength concrete has a compressive strength of 62,3 MPa, which has exceeded the minimum compressive strength required for high strength concrete, which is 41.4 MPa. The concrete density and compressive strength after curing time of 28 days are illustrated in Fig. 3.

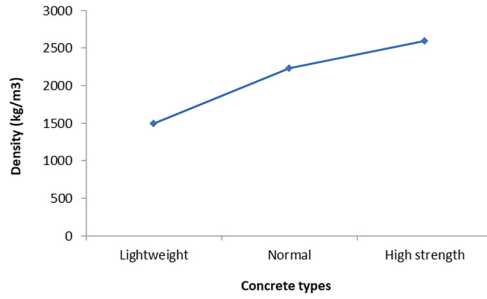
### 3.3 Strength Development

The results of the compressive strength test show an important growth in the compressive strength of the concrete along with the rise of the concrete ages even though the concrete has passed the age of 28 days. For normal concrete, the greatest compressive strength is achieved at the age of 90 days reaching 34,91 MPa. Meanwhile, for at the age of concrete 3 days, 7 days, 14 days, 21 days, 28 days and 56 days, the compressive strength are 19,72 MPa; 22,84 MPa; 26,80 MPa; 29,44 MPa; 32,65 MPa; and 33,88 MPa respectively.

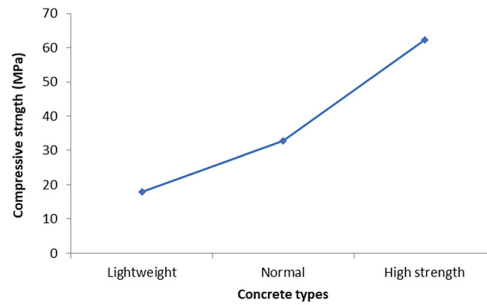
The same trend is also found in lightweight concrete and high strength concrete. In high strength concrete, there is a strength development at concrete ages of 3 days, 7 days, 14 days, 21 days, 28 days, 56 days and 90 days at 45,20 MPa; 50,58 MPa; 54,54 MPa; 57,56 MPa; 62,28 MPa; 65,30 MPa; and 69,45 MPa respectively. Furthermore, in lightweight concrete, the strength development occurred at variations in concrete curing times of 3 days, 7 days, 14 days, 21 days, 28 days, 56 days and 90 days at 8,49 MPa; 10,29 MPa; 13,40 MPa; 15,85 MPa; 17,74 MPa; 18,59 MPa; and 18,97 MPa respectively. In the case of lightweight concrete strength development after 28 days it does not show a significant increase, but in high-strength concrete there was still a significant increase in strength. Figure 4 shows strength development in each concrete type.

### 3.4 Assessment Factor

When the compressive strength of concrete at the age of 28 days is normalized to a value of 1, a comparison of the compressive strength at a certain age is obtained which

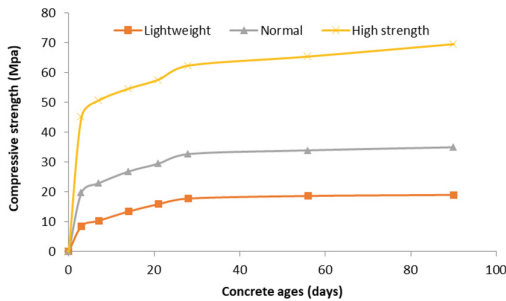


(a)



(b)

**Fig. 3.** Density (a) and compressive strength (b) of each concrete type after 28 days curing time.



**Fig. 4.** Strength development in each concrete type.

is called the assessment factor. The assessment factor on lightweight concrete for ages of 3, 7, 14, 21, 56, and 90 days, respectively, was 0,48; 0,58; 0,76; 0,89; 1,05; and 1,07. Meanwhile, for normal concrete, the assessment factors are 0,6; 0,70; 0,82; 0,90; 1,05; and 1,07 for concrete ages of 3, 7, 14, 21, 56, and 90 days respectively. Furthermore, the assessment factor for high-strength concrete showed values of 0,73; 0,81; 0,88; 0,92; 1,06; and 1,12 respectively for concrete ages of 3, 7, 14, 21, 56, and 90 days.

Figure 5 shows the comparison of assessment factors among the concrete types. It can be seen that before the age of 28 days, lightweight concrete has the lowest assessment factor but after 28 days of age, the assessment factor of lightweight concrete equals

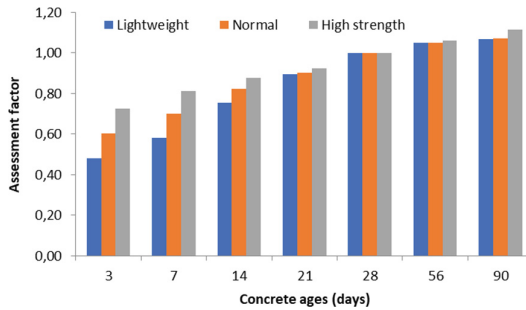


Fig. 5. Concrete assessment factor.

the assessment factor value of normal concrete. Among the three types of concrete, high-strength concrete has the highest assessment factor, where the assessment factor continues to increase sharply even though the concrete reaches 90 days old. The role of silica fume which is able to bind calcium hydroxide so as to provide a continuous pozzolanic reaction as the result the concrete strength continue to increase significantly after the concrete is 28 days old [9, 10, 16–18].

### 3.5 Assessment Factor Comparison to the Codes

Figure 6 illustrates the comparison of the assessment factor between the concrete codes: PBI NI-2 1971, SNI 03-6805-2002, and the experimental results for normal concrete because the codes do not accommodate other types of concrete. In general, it appears that the assessment factors from the three sources are not much different. The biggest difference is in the early age especially at 3 days where the difference is around 30% in the assessment factor between PBI NI-2 1971 and the experimental results. This is assumed to be due to the use of different types of cement. The experiment used Pozzolan Portland Cement or PPC because currently, this type of cement is commercialized in the market. This PPC contains higher calcium tri silicate hydrate or  $C_3S$  element so that it produces higher heat of hydration in the early age; as a result high compressive strength is achieved at the beginning [19]. As for PBI NI-2 1971, it is assumed to use Ordinary Portland Cement (OPC) because this code is outdated and at that time the availability of cement type in the market was OPC.

For compressive strength above 28 days, which is 90 days, the experimental results show a lower assessment factor because the  $C_3S$  component does not provide a high final strength. Furthermore, SNI 03-6805-2002 does not provide an assessment factor for the concrete age of 14 and 21 days; there for e in Fig. 6 only shows the assessment factor from PBI NI-2 1971 and the experimental result for those concrete ages. The assessment factor of the two is found not much different.

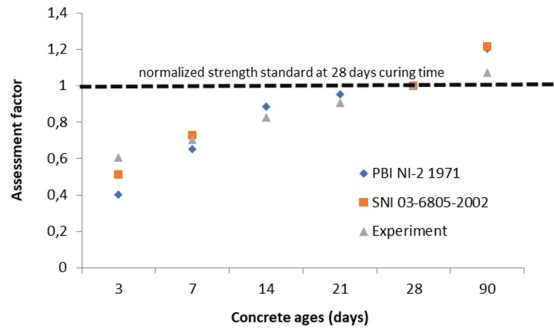


Fig. 6. Assessment factor comparison.

## 4 Conclusion

This paper discusses the assessment factor of strength development for high strength, normal, and lightweight concrete. The assessment factor on lightweight concrete for ages of 3, 7, 14, 21, 56, and 90 days, respectively, was 0,48; 0,58; 0,76; 0,89; 1,05; and 1,07. Meanwhile, for normal concrete, the assessment factors are 0,6; 0,70; 0,82; 0,90; 1,05; and 1,07 for concrete curing times of 3, 7, 14, 21, 56, and 90 days respectively. Furthermore, the assessment factor for high-strength concrete showed values of 0,73; 0,81; 0,88; 0,92; 1,06; and 1,12 respectively for concrete ages of 3, 7, 14, 21, 56, and 90 days.

Among the three types of concrete, high-strength concrete has the highest assessment factor, where the assessment factor continues to increase sharply even though the concrete reaches 90 days old.

The assessment factor between the concrete codes: PBI NI-2 1971, SNI guideline 03-6805-2002, and the experimental results for normal concrete shows that in general, it appears that the assessment factors from the three sources are not much different.

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